1. COORDINATION:

CONTRACTOR TO COORDINATE ALL PIPE AND CONDUIT LOCATIONS WITH MECHANICAL AND ELECTRICAL DRAWINGS PRIOR TO PLACING CONCRETE. CONTRACTOR SHALL PROVIDE SHOP DRAWINGS FOR EQUIPMENT AND ANCHOR BOLT LOCATIONS.

STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH MECHANICAL AND ELECTRICAL DRAWINGS TO PROPERLY LOCATE WALL PIPES, PIPE SLEEVES, ANCHOR BOLTS, BLOCKOUTS, ETC. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE COUNTY ENGINEER BEFORE PROCEEDING WITH THE WORK.

2. PRECAUTIONS:

CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO PREVENT FLOTATION OF STRUCTURES UNTIL FULLY CONSTRUCTED AND BACKFILL IS IN PLACE AND COMPACTED.

3. DESIGN CRITERIA AND LOADS:

ACI 350	CONCRETE SANITARY ENGINEERING STRUCTURES
ACI 318	BUILDING CODE REQUIREMENTS FOR REINFORCED
	CONCRETE
ASTM C 478	STANDARD SPECIFICATION FOR CIRCULAR PRECAST
	REINFORCED CONCRETE MANHOLE SECTIONS
ASTM C 433	STANDARD SPECIFICATION FOR JOINTS FOR CONCRETE
	PIPE AND MANHOLES, USING RUBBER GASKETS

DESIGN LIVE LOADS:

WET WELL TOP SLAB SITE PAD - VEHICULAR AREA BEARING SITE PAD - NON-VEHICULAR AREA BEARING AASHTO HS20-44. AASHTO HS20-44. 300 PSF.

NET ALLOWABLE SOIL BEARING CAPACITY: DESIGN ENGINEER TO CONFIRM 1500 PSF. (MINIMUM)

4. CAST-IN-PLACE CONCRETE:

CAST-IN-PLACE CONCRETE SHALL HAVE THE FOLLOWING MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS:

SLABS ON GRADE4,000 PSIPIPE SUPPORTS, PUMPS PADS, ENCASEMENTS4,000 PSI

5. PRECAST CONCRETE:

PRECAST WET WELL CONCRETE SHALL HAVE MINIMUM 4,000 PSI COMPRESSIVE STRENGTH AT 28 DAYS.

6. REINFORCING STEEL:

REINFORCING STEEL FOR ALL BARS SHALL CONFORM TO ASTM 615, GRADE 60 OF UNITED STATES MANUFACTURE.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

7. REINFORCEMENT CLEARANCE:

CLEARANCE OF REINFORCING STEEL FROM THE FACE OF CONCRETE TO THE OUTERMOST TIE OR BAR SHALL BE 2", UNLESS OTHERWISE NOTED ON THE DRAWINGS.

SCALE		REVISIONS		λ	PROJECT No.: FILE No.:		
AS SHOWN	1 No.	ADDED 2 BLACK & VEATCH FALL HAZARD PROTECTION SHTS 8/31/2 DESCRIPTION AP	3 V'D.	HILLSBOROUGH COUNTY PUBLIC UTILITIES DEPARTMENT 925 E. TWIGGS STREET / TAMPA, FLORIDA 33602	DESIGNED BY: DRAWN BY: CHECKED BY:	RMA TRS DAW & WNH OCTOBER 2023 AS SHOWN	NOTES STRUCTURAL

OPENINGS SHALL BE LOCATED BASED ON THE REQUIREMENTS SPECIFIED IN THE MECHANICAL DRAWINGS. MANUFACTURER RESPONSIBLE FOR	
COORDINATING SIZE AND LOCATION WITH ENGINEER.	
9. ALUMINUM ACCESS HATCH:	
ACCESS HATCH COVER SHALL BE ALUMINUM, MEET HILLSBOROUGH	
COUNTY SPECIFICATIONS AND BE SIZED AND DETAILED TO MEET THE REQUIREMENTS OF THE SELECTED PUMPS.	
10. WATERSTOPS:	
WATERSTOPS SHALL BE DUMBELL STYLE, MADE OF FLEXIBLE PVC AND	
MEASURING 6" x 3/8".	
11. COATINGS:	
THE WET WELL INTERIOR SHALL RECEIVE A FIELD APPLIED CORROSION RESISTANT COATING AS PER HILLSBOROUGH COUNTY SPECIFICATIONS.	
12. SHOP DRAWINGS:	
THE FOLLOWING SHOP DRAWINGS SHALL BE SUBMITTED FOR REVIEW.	
FABRICATION SHALL NOT COMMENCE UNTIL ALL REVIEWS ARE	
COMPLETED.	
- REINFORCING STEEL - ANCHOR BOLTS FOR PUMPS	
- PRECAST CONCRETE - CONCRETE MIX DESIGNS - GROUTS - ACCESS HATCHS AND FRAMES	
- WATERPROOF JOINTS - EXPANSION JOINT MATERIAL	
- LINKSEAL STYLE CONNECTION - FLEXIBLE 'BOOT' CONNECTIONS	
- GASKETS - WATERSTOPS	
13. FOUNDATIONS:	
REMOVE ALL ORGANIC SOIL, CLAYS AND OTHER COMPRESSIBLE	
MATERIALS.	
A MINIMUM OF 6 INCHES OF COMPACTED CRUSHED STONE (SEE	
HILLSBOROUGH COUNTY STANDARDS) SHALL BE PLACED UNDER THE WET	
WELL BASE SLAB AND DROP CONNECTION.	
DEWATER EXCAVATION DURING WET WELL INSTALLATION. ALL WORK SHALL BE DONE IN THE "DRY".	
14. DESIGN CONDITIONS VS SITE CONDITIONS:	
THE WET WELL DESIGN WALL THICKNESS, BOTTOM SLAB THICKNESS AND	
DIMENSIONS, BOTTOM SLAB EXTENSION, AND FOUNDATION BASE COURSE	
ARE MINIMUM DIMENSIONS. THE STRUCTURAL DESIGN IS BASED ON THE LOADS AND CONDITIONS LISTED HERE. TO USE THESE PLANS AS-IS, THE	
ENGINEER SHALL VERIFY THAT THE SITE CONDITIONS MEET THE DESIGN	
CONDITIONS, INCLUDING THE GEOTECHNICAL CONDITIONS AND FLOTATION	
CALCULATIONS. IF THE SITE CONDITIONS VARY FROM THE DESIGN CONDITIONS, THE ENGINEER SHALL MODIFY THE DESIGN AS NEEDED AND	
PROVIDE SIGNED AND SEALED DRAWINGS TO THE COUNTY FOR	
APPROVAL.	

WET WELL CONSTRUCTION NOTES:

GRAVITY PIPE, DISCHARGE PIPING, AUXILIARY SUCTION AND PROCESS PIPING SIZES TO BE DETERMINED BY THE ENGINEER. REFER TO MECHANICAL DRAWINGS.

OPENING REQUIREMENTS:

GRAVITY PIPE:	CAST-IN-PLACE OR CORED OPENING
	FOR FLEXIBLE 'BOOT' CONNECTION.
DISCHARGE PIPING:	LINK SEAL TYPE PIPE SEAL.
AUXILIARY PIPING:	LINK SEAL TYPE PIPE SEAL.
OTHER PIPING:	NON-SHRINK GROUT.
OTHER PIPING:	NON-SHRINK GROUT.

DROP CONNECTION SHALL BE USED WHEN THE ELEVATION DIFFERENCE BETWEEN THE INVERT OF THE INFLUENT PIPE AND THE PUMP LOW WATER LEVEL IS GREATER THAN 2 FEET.

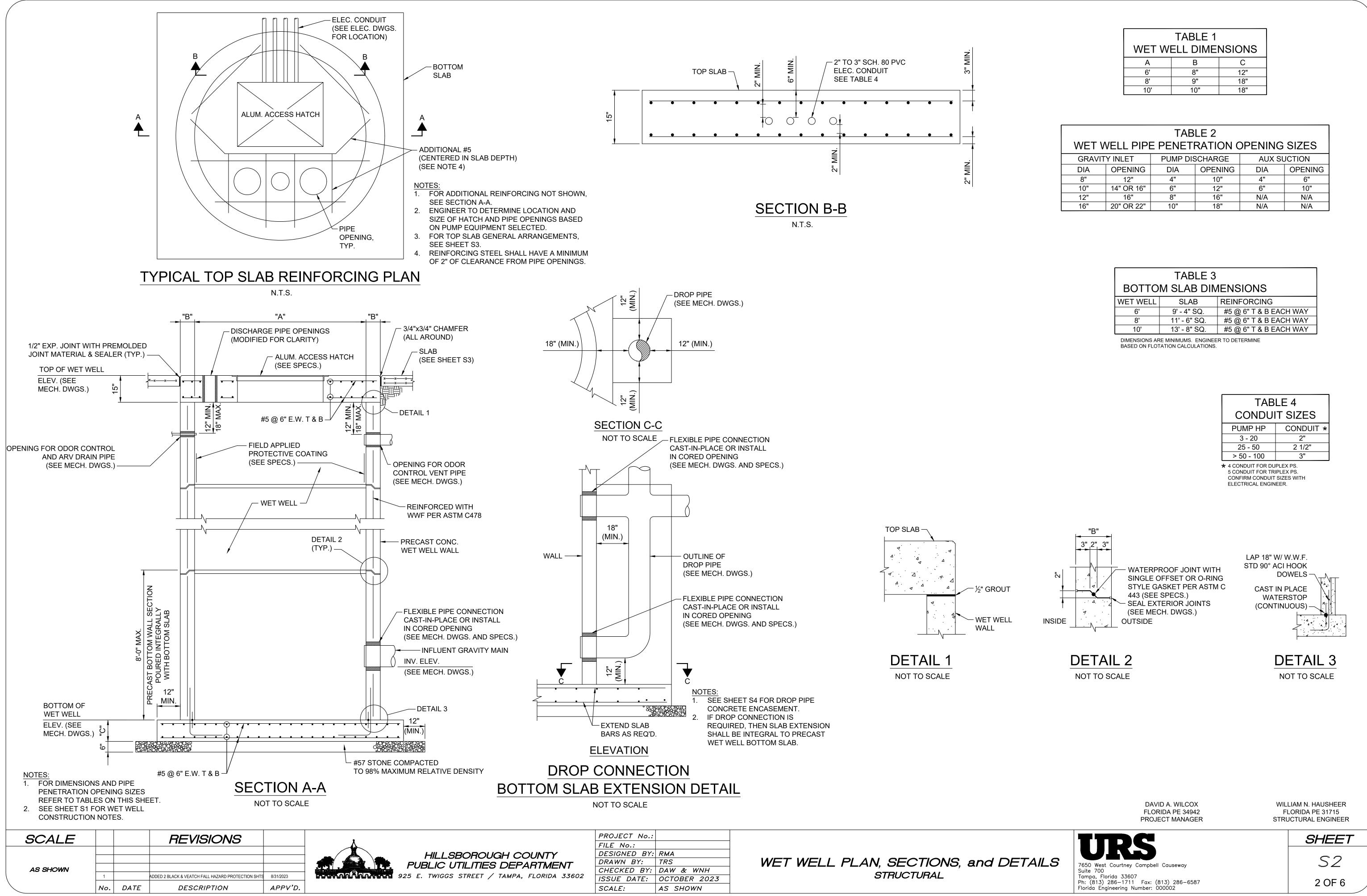
WET WELL BARREL JOINTS SHALL BE SEALED WITH A SINGLE OFFSET OR O-RING STYLE GASKET PER ASTM C433.

TOP SLAB, BOTTOM SLAB AND WET WELL BARREL SHALL BE PRECAST.

E OF DRAWINGS:

ICCESSOR PROFESSIONAL ENGINEER UTILIZING THESE RAWINGS MUST ABIDE BY THE RULES AND REGULATIONS ONTAINED IN 61G15-27.001 OF THE FLORIDA OMINISTRATIVE CODE.

DAVID A. WILCOX FLORIDA PE 34942 PROJECT MANAGER	FI	LIAM N. HAUSHEER LORIDA PE 31715 JCTURAL ENGINEER
TRS		SHEET
7650 West Courtney Campbell Causeway Suite 700		S 1
Tampa, Florida 33607 Ph: (813) 286-1711 Fax: (813) 286-6587 Florida Engineering Number: 000002		1 OF 6

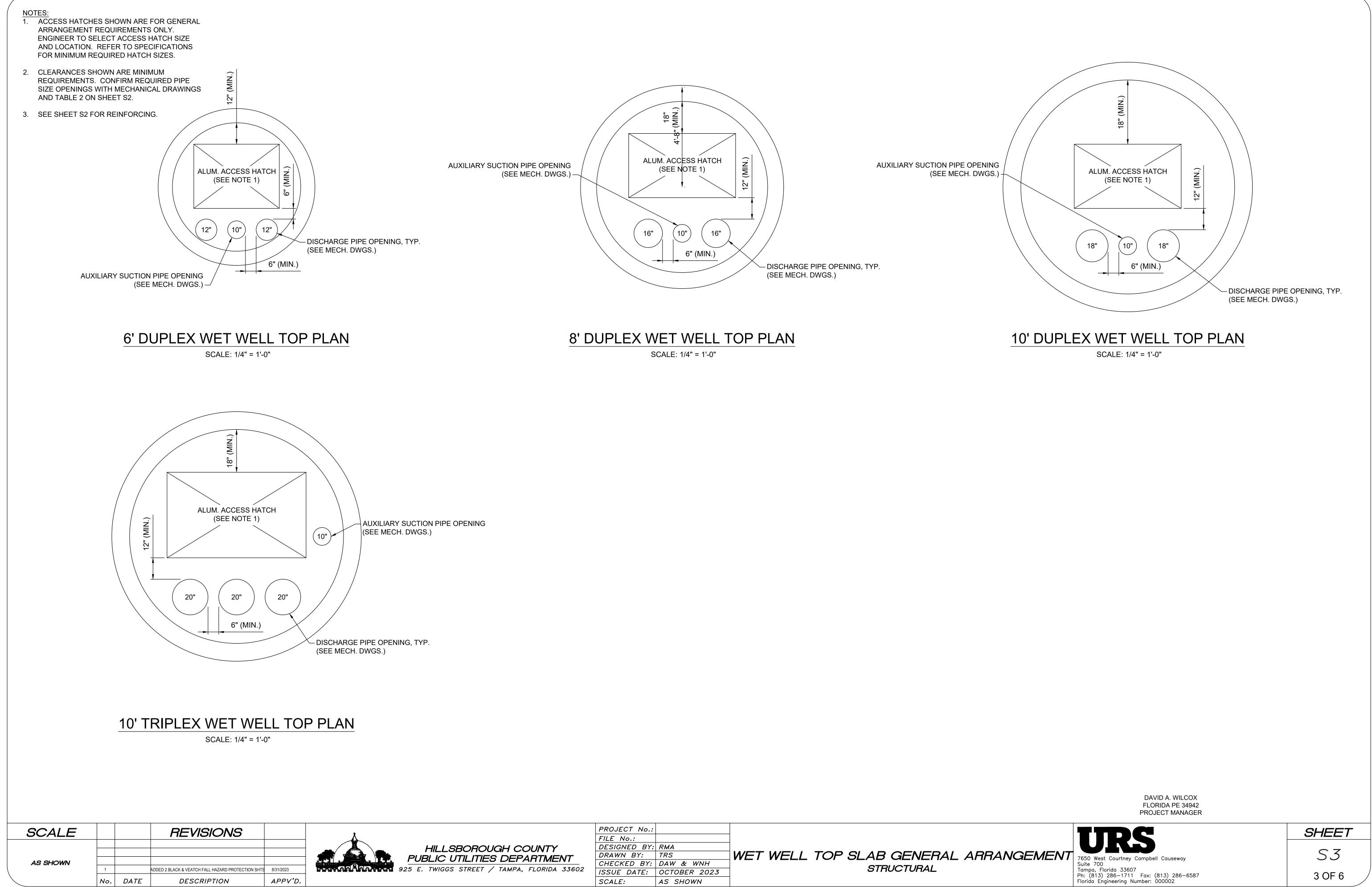


WET WI	TABLE 1 ELL DIMEI	NSIONS
A	В	С
6'	8"	12"
8'	9"	18"
10'	10"	18"

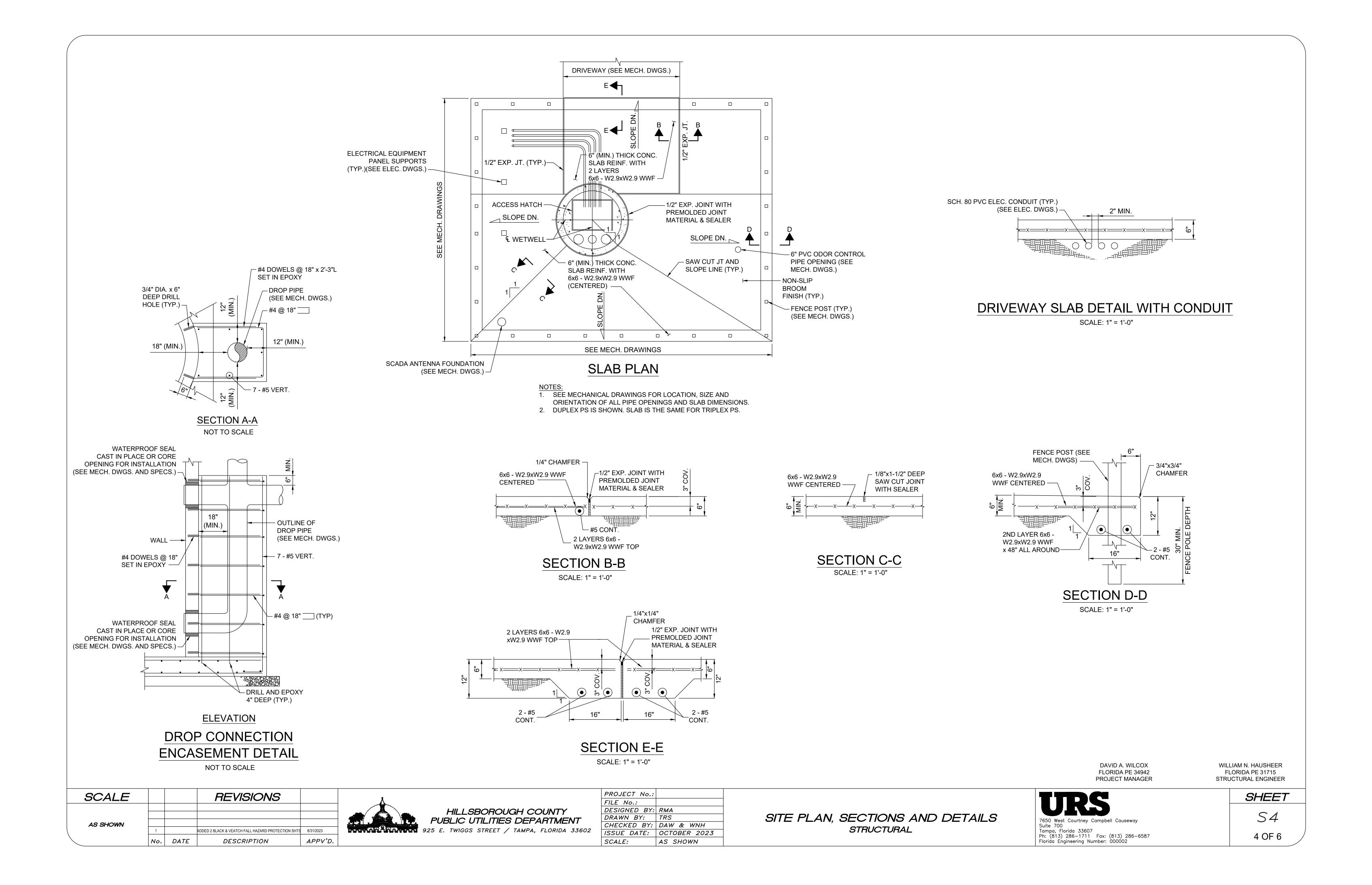
TABLE 2							
WET V	WET WELL PIPE PENETRATION OPENING SIZES						
GRAVITY INLET		PUMP DISCHARGE		AUX SUCTION			
DIA	OPENING	DIA	OPENING	DIA	OPENING		
8"	12"	4"	10"	4"	6"		
10"	14" OR 16"	6"	12"	6"	10"		
12"	16"	8"	16"	N/A	N/A		
16"	20" OR 22"	10"	18"	N/A	N/A		

TABLE 3						
BOTTO	M SLAB DIN	IENSIONS				
WET WELL	SLAB	REINFORCING				
6'	9' - 4" SQ.	#5 @ 6" T & B EACH WAY				
8'	11' - 6" SQ.	#5 @ 6" T & B EACH WAY				
10'	13' - 8" SQ.	#5 @ 6" T & B EACH WAY				

	TAB CONDUI				
	PUMP HP	CONDUIT *			
	3 - 20	2"			
	25 - 50	2 1/2"			
	> 50 - 100	3"			
1	★ 4 CONDUIT FOR DUPLEX PS.				



	PROJECT No.:				
	FILE No.:				
SBOROUGH COUNTY	DESIGNED BY:	RMA			
UTILITIES DEPARTMENT IS STREET / TAMPA, FLORIDA 33602	DRAWN BY:	TRS	WET WELL TOP SLAB GENERA	. TOP SLAB GENERAL AI	ARF
	CHECKED BY:	DAW & WNH			
	ISSUE DATE:	OCTOBER 2023		SINUCIUNAL	
	SCALE:	AS SHOWN			



		GENERAL	
	1.	THE APPLICABLE BUILDING CODE IS THE 2015 INTERNATIONAL BUILDING CODE (IBC) AND THE 2017 FLORIDA BUILDING CODE, SIXTH EDITION.	
	2.	THE REQUIREMENTS INDICATED ON THIS SHEET ARE INTENDED AS A BASIC SUMMARY OF THE MATERIAL AND CONSTRUCTION REQUIREMENTS FOR THE PROJECT. ADDITIONAL REQUIREMENTS ARE PROVIDED IN THE DRAWINGS AND ALL WORK MUST BE IN ACCORDANCE WITH HILLSBOROUGH COUNTY STANDARDS AND SPECIFICATIONS.	
		ALL STRUCTURAL RELATED SHOP DRAWINGS SHALL BE REVIEWED BY THE COUNTY PRIOR TO CONSTRUCTION.	
	4.	CONTRACTOR SHALL INSTALL AND COMPACT CRUSHED STONE AS INDICATED ON THE DRAWINGS. AFTER COMPACTION BUT PRIOR TO INSTALLATION OF REINFORCING STEEL AND CONCRETE THE COUNTY WILL VERIFY COMPACTION REQUIREMENTS HAVE BEEN SATISFIED. PROVIDE 48 HOURS NOTICE PRIOR TO SUBGRADE BEING READY FOR COMPACTION VERIFICATION.	
		CONCRETE	
	1.	SEE BELOW FOR CAST-IN-PLACE AND PRECAST CONCRETE STRENGTH REQUIREMENTS.	
	2.	CONCRETE SHALL BE CONTROLLED WITHIN THE FOLLOWING LIMITING REQUIREMENTS:	
		- CONCRETE STRENGTH - 4,000 PSI MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS.	
		- COARSE AGGREGATE - MAXIMUM NOMINAL COARSE AGGREGATE SIZE, 1 INCH.	
		- CONSISTENCY - WORKABLE, WITHOUT SEGREGATION, WITH SLUMP NOT MORE THAN 5 INCHES WHEN CONCRETE IS PLACED.	
	3.	THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE COUNTY FOR REVIEW:	
		- MANUFACTURER DATA FOR CONCRETE MIX - PROPOSED MIXTURE PROPORTIONS	
		- CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS	
	4.	SPECIFIED OR SHOWN ON THE PLANS, SHALL BE ACCEPTABLE TO THE COUNTY PRIOR TO PLACING	
		CONCRETE.	Â
		REINFORCING STEEL	
	1.	ALL REINFORCING BARS SHALL BE GRADE 60, DEFORMED, ASTM A615, UNLESS NOTED OTHERWISE.	
	2.	DIMENSIONS TO REINFORCING BARS ARE TO BAR CENTERLINES UNLESS NOTED OTHERWISE. BAR COVER IS THE CLEAR DISTANCE BETWEEN THE BAR AND THE CONCRETE SURFACE.	
	3.	CONCRETE COVER SHALL BE 2" WITH EXCEPTION OF LOCATIONS WHERE CONCRETE IS PLACED AGAINST SOIL. WHERE CONCRETE IS PLACED AGAINST SOIL, PROVIDE 3" OF CONCRETE COVER.	
	4.	NO WELDING OF REINFORCING BARS SHALL BE PERMITTED UNLESS APPROVAL IS OBTAINED FROM THE COUNTY PRIOR TO CONSTRUCTION.	
			<u>EAP</u> JPS
8/31/23	REN	USED FOR COUNTY STANDARDS	EAP JPS EB RAZ

STRUCTURAL NOTES

POST-INSTALLED ANCHORS

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\Lambda N/A

- 1. POST-INSTALLED ANCHORS SHALL INCLUDE ADHESIVE ANCHORS (THREADED RODS, BOLTS OR REINFORCING BARS), EXPANSION ANCHORS, AND UNDERCUT ANCHORS INSTALLED INTO HARDENED CONCRETE. SEE DETAILS FOR SIZE AND DIAMETER OF ANCHORS.
- 2. CARE SHALL BE TAKEN TO AVOID CONFLICTS WITH EXISTING REINFORCING STEEL AND OTHER EMBEDDED ITEMS WHEN DRILLING HOLES. REINFORCING BARS SHALL NOT BE DAMAGED DURING DRILLING OR ANCHOR INSTALLATION. HOLES SHALL BE DRILLED AND CLEANED PER THE PRODUCT MANUFACTURER'S INSTRUCTIONS. ANCHORS SHALL BE INSTALLED PER THE PRODUCT MANUFACTURER'S INSTRUCTIONS AT NOT LESS THAN MINIMUM EDGE DISTANCES AND/OR SPACING INDICATED IN THE MANUFACTURER'S LITERATURE.
- 3. DATA, CATALOG CUTS, AND MANUFACTURER'S RESEARCH REPORTS (FROM INDEPENDENT ORGANIZATIONS SUCH AS ICC-ES OR IAPMO UES) INDICATING THE MANUFACTURER AND TYPES OF ADHESIVE ANCHORS, EXPANSION ANCHORS, AND UNDERCUT ANCHORS TO BE SUPPLIED SHALL BE SUBMITTED. PRODUCTS SHALL BE SINGLE COMPONENT ANCHORS TESTED IN ACCORDANCE WITH ICC ACI193 (EXPANSION ANCHORS) AND ACI 308, AND SHALL HAVE MANUFACTURER'S RESEARCH REPORT IN COMPLIANCE WITH THE APPLICABLE BUILDING CODE. THE ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS AND ALL APPLICABLE REQUIREMENTS OF THE MANUFACTURER'S RESEARCH REPORT, AND SHALL BE APPROVED FOR USE IN CRACKED CONCRETE. ANCHORS SHALL BE INSTALLED IN DRY CONDITIONS, AFTER CONCRETE HAS REACHED ITS 21 DAY STRENGTH, HAMMER DRILLED HOLE ONLY, AND UNDER THE CONCRETE TEMPERATURE MAXIMUMS OF 162 F (SHORT-TERM) AND 110 F (LONG-TERM).
- 4. SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE LISTED HEREIN OR INDICATED ON THE DRAWINGS SHALL BE SUBMITTED TO COUNTY FOR REVIEW AND APPROVAL. PRODUCT ICC-ESR EVALUATION REPORTS SHALL BE INCLUDED WITH THE SUBMITTAL PACKAGE. IF REQUESTED. CALCULATIONS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE OF FLORIDA, USING METHODS AND PROCEDURES REQUIRED BY THE BUILDING CODE MAY BE REQUIRED AS PART OF THE SUBMITTAL PACKAGE.
- 5. INSPECTION WILL BE PROVIDED BY THE COUNTY FOR ALL POST-INSTALLED ANCHORS.

EXISTING STRUCTURES

- 1. DRAWING S-02 DEPICTS WORK AT EXISTING STRUCTURES. ALL DIMENSIONS AND ALL DEPICTIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS, STARTING FABRICATION, OR STARTING CONSTRUCTION.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE, REPAIRS OR STRUCTURAL MODIFICATIONS THAT ARE REQUIRED DUE TO DEMOLITION BEYOND THE LIMITS IDENTIFIED ON THE DBAWINGS.
- 3. REINFORCEMENT FOR ANY EXISTING CONCRETE ELEMENT SHALL NOT BE DAMAGED UNLESS THE ELEMENT IS TO BE DEMOLISHED AS NOTED ON THE DRAWINGS. WHEN LOCATING EXISTING REINFORCEMENT IS REQUIRED, IT SHALL BE LOCATED USING NON-DESTRUCTIVE METHODS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE, REPAIRS OR STRUCTURAL MODIFICATIONS THAT ARE REQUIRED DUE TO DAMAGE OF CONCRETE, MASONRY OR REINFORCEMENT THAT HAS BEEN IDENTIFIED ON THE DRAWINGS TO REQUIRE FIELD VERIFICATION.
- 4. CORE DRILLING AND SAW CUTTING SHALL NOT BE PERFORMED UNLESS INDICATED ON THE DRAWINGS OR APPROVED BY THE ENGINEER.
- 5. EXPOSED CONCRETE SURFACES THAT REMAIN AFTER DEMOLITION SHALL BE REPAIRED TO MATCH ADJACENT CONCRETE SURFACES.
- 6. UNLESS OTHERWISE INDICATED ON THE DRAWINGS, EXPOSED CONCRETE SURFACES WITH REINFORCEMENT, ANCHOR BOLTS, HANGER RODS, OR OTHER EXPOSED METAL EMBEDMENTS SHALL BE REPAIRED BY CUTTING OFF THE METAL AT THE FACE OF THE CONCRETE. GRINDING SMOOTH. AND COATING WITH A CORROSION INHIBITOR, SIKA ARMATEC 110 EpoCem 111 OR EQUAL. COATING SHALL EXTEND A MINIMUM OF 1" BEYOND THE EDGE OF ANY EXPOSED METAL.

NEW STRUCTURES

- 1. WHERE ANCHOR SYSTEMS ARE TO BE INSTALLED WITH NEW CONSTRUCTION, PEDESTAL MOUNTED ANCHOR POINTS SHALL BE THE PRIMARY OPTION UNLESS INGRESS/EGRESS AT A SITE OR ACCESS TO EQUIPMENT IS OF CONCERN. FLUSH MOUNTED ANCHOR POINTS MAY BE USED WITH SPECIFIC APPROVAL FROM THE COUNTY. CONTRACTOR MAY USE PRECAST OR CAST IN-PLACE PEDESTAL OPTION. SEE DETAILS FOR DOWEL ARRANGEMENT FOR EACH OPTION.
- 2. SITE LAYOUT SHALL ACCOUNT FOR HOIST CONNECTION LOCATION(S) TO ALLOW FOR FULL ACCESS TO ALL ELECTRICAL EQUIPMENT, WETWELL HATCH OPENING, AND THE STAGING OF PORTABLE DIESEL PUMPS OR OTHER PERTINENT APPURTENANCES REQUIRED BY MAINTENANCE STAFF. IT IS EXPECTED THAT A MINIMUM OF TWO HOIST CONNECTION LOCATIONS WILL BE REQUIRED PER SITE.

IGINEER OF RECORD:						
IICHOLAS W. ECKHARDT, P.E.						
ORIDA LICENSE NO.:						
9144						

BLACK & VEATCH Building a world of difference:

Black & Veatch Corporation 3405 W. Dr. M.L. King Jr. Blvd, Suite 125 Tampa, Florida 33607 Certificate No. 8132



1. THE SCOPE OF THESE CONSTRUCTION DOCUMENTS IS LIMITED TO THE CONSTRUCTION OF THE FALL PROTECTION SYSTEM ANCHOR POINTS AND SLEEVE ANCHORAGE. ALL OTHER COMPONENTS OF THE FALL PROTECTION SYSTEM SHALL BE USED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDED SPECIFICATIONS.

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FALL PROTECTION SYSTEM

2. THE DESIGN PROVIDED IN THESE CONSTRUCTION DOCUMENTS SHALL BE USED AS A GENERAL GUIDELINE FOR THE PLACEMENT AND CONSTRUCTION OF THE ANCHOR POINTS AND SLEEVE ANCHORAGE OF THE FALL PROTECTION SYSTEM. CONFIGURATIONS AND ORIENTATION MAY VARY FOR SPECIFIC SITES. IN ALL CASES, CONTRACTOR SHALL FOLLOW THE MINIMUM REQUIREMENTS ESTABLISHED IN THE DRAWINGS.

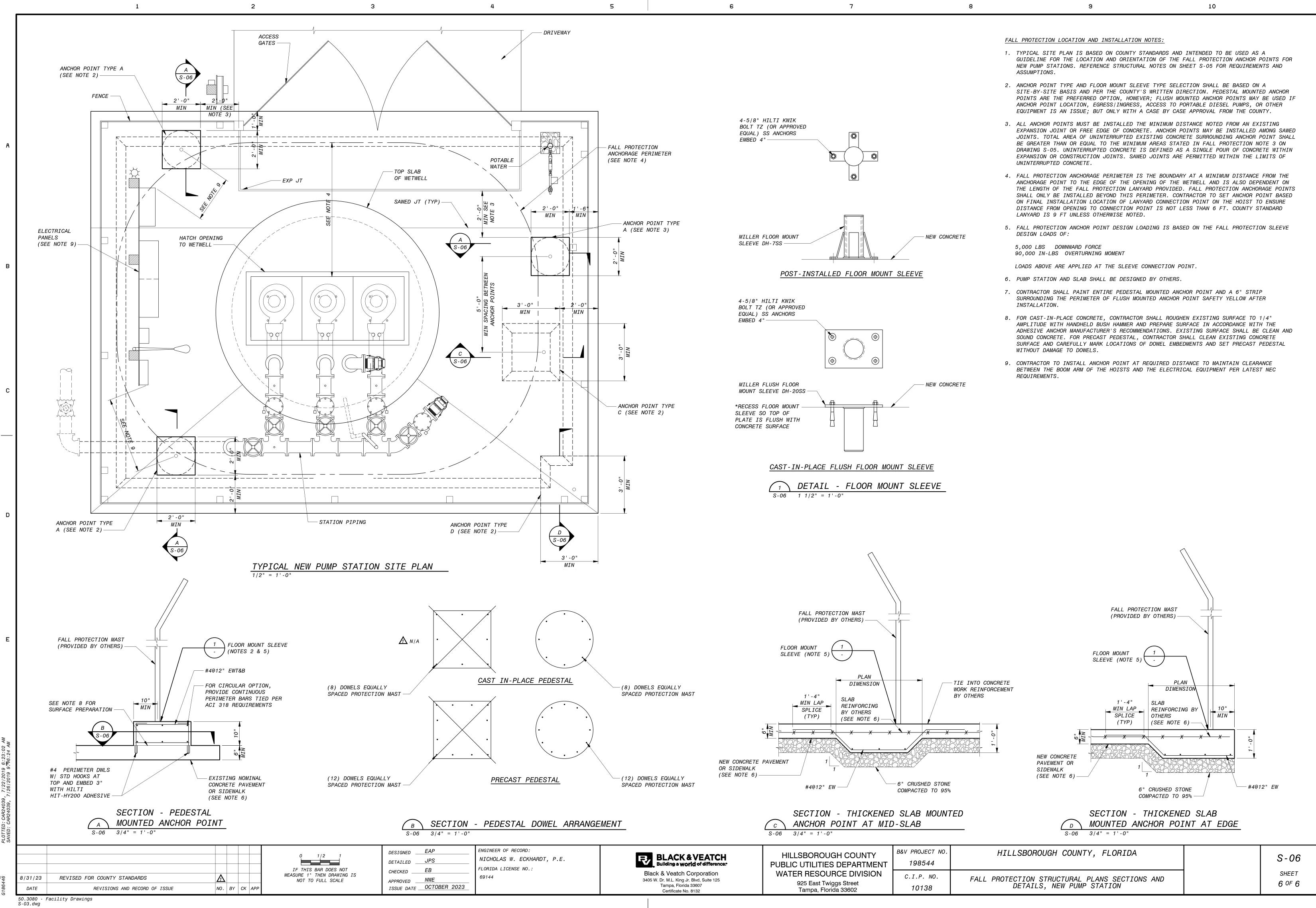
3. THE FALL PROTECTION ANCHOR DESIGN ASSUMES EXISTING CONCRETE WITH THE MINIMUM REQUIREMENTS LISTED BELOW.

		MIN AREA OF EXISTING
CONCRETE STRENGTH	MIN THICKNESS	UNINTERRUPTED CONCRETE
4000 PSI	6"	36 SQ FT
4000 PSI	<i>4</i> "	50 SQ FT
3000 PSI	6 "	50 SQ FT
3000 PSI	<i>4</i> "	75 SQ FT

UNINTERRUPTED CONCRETE IS DEFINED AS A SINGLE POUR OF CONCRETE WITHIN EXPANSION OR CONSTRUCTION JOINTS. SAWED JOINTS ARE PERMITTED WITH THE LIMITS OF UNINTERRUPTED CONCRETE.

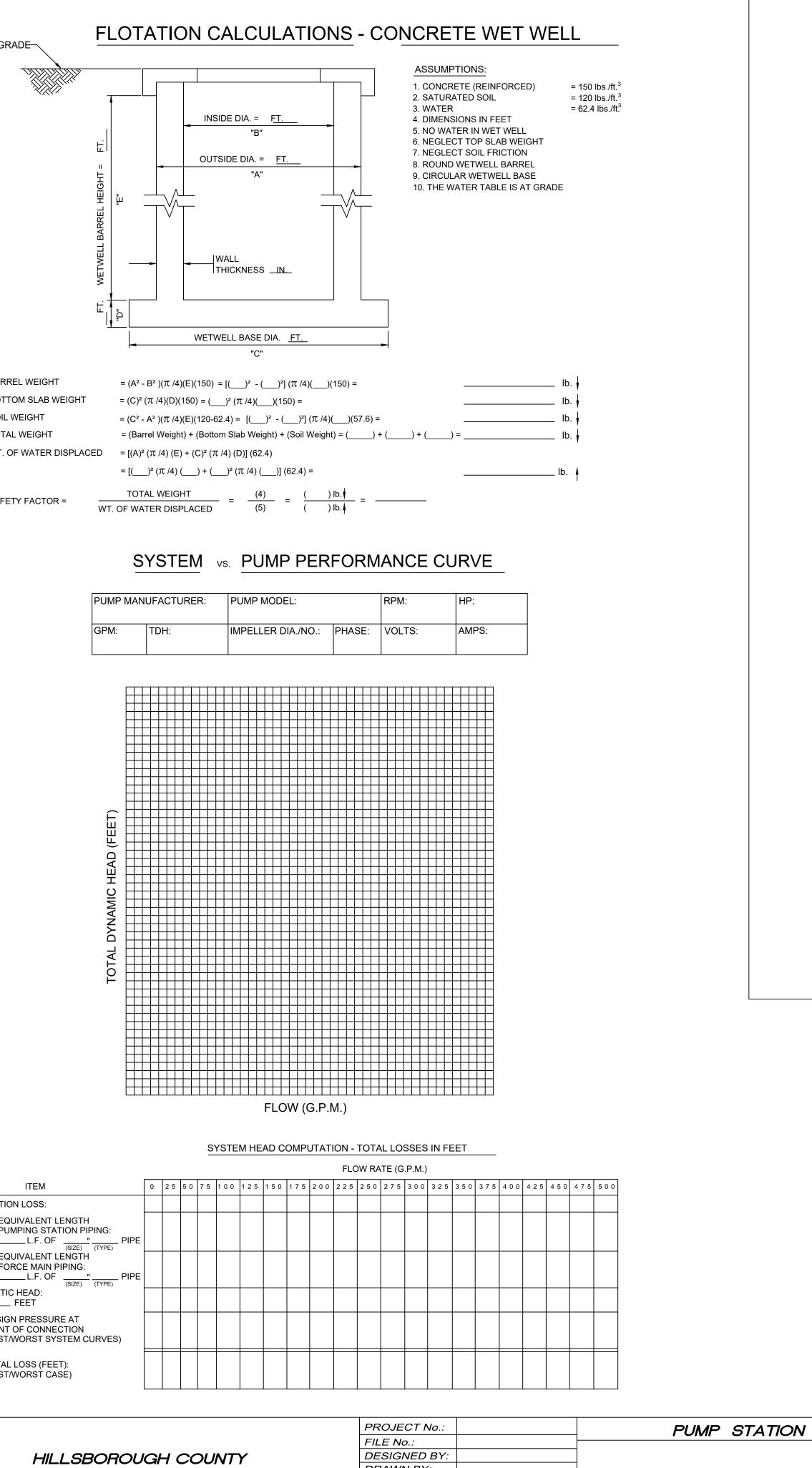
IF CONDITIONS DIFFER THAN THESE MINIMUMS, THE CONTRACTOR IS TO CONTACT THE COUNTY AND A SITE SPECIFIC DESIGN WILL NEED TO BE DONE TO ENSURE SAFE INSTALLATION OF THE FALL PROTECTION ANCHOR.

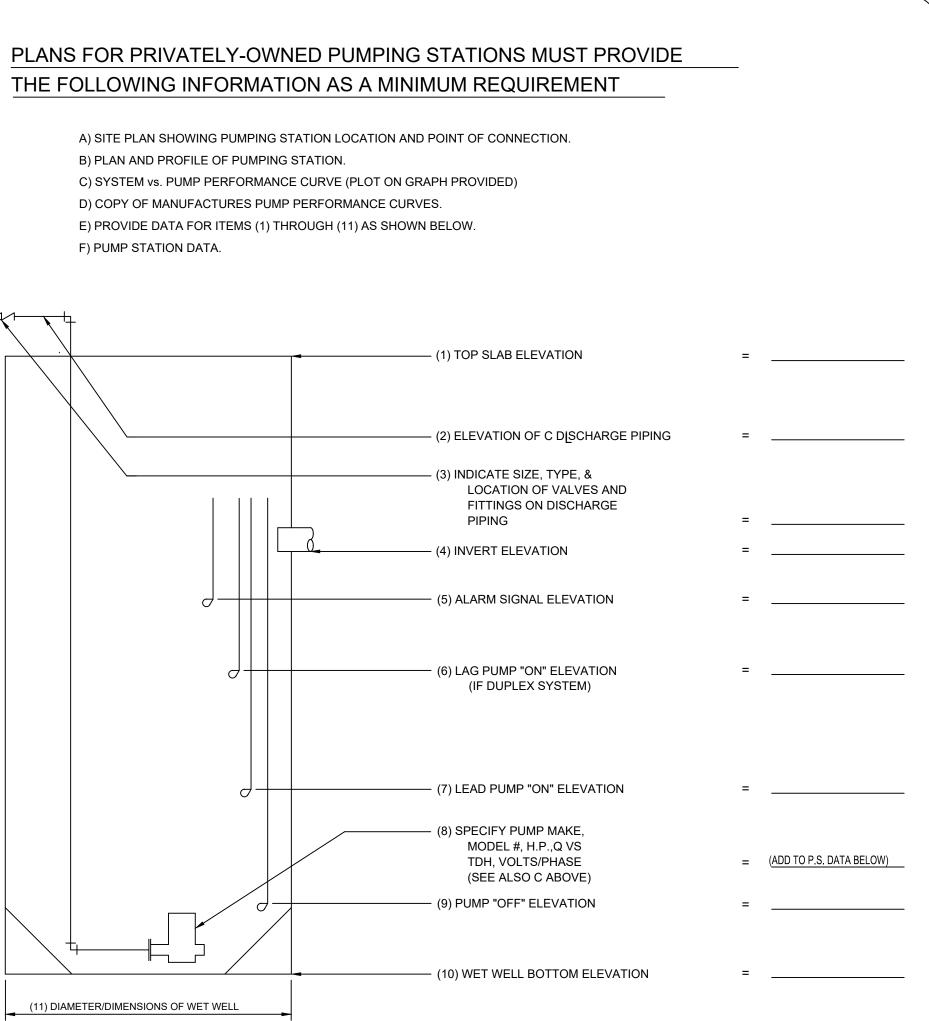
B&V PROJECT NO. 198544	HILLSBOROUGH COUNTY, FLORIDA	S-05
C.I.P. NO.	FALL PROTECTION STRUCTURAL	SHEET
10138	GENERAL NOTES	5 OF 6



<u>F</u> A	ALL PROTECTION LOCATION AND INSTALLATION NOTES:
1.	TYPICAL SITE PLAN IS BASED ON COUNTY STANDARDS AND INTENDED TO BE USED AS A GUIDELINE FOR THE LOCATION AND ORIENTATION OF THE FALL PROTECTION ANCHOR POINTS FOR NEW PUMP STATIONS. REFERENCE STRUCTURAL NOTES ON SHEET S-05 FOR REQUIREMENTS AND ASSUMPTIONS.
2.	ANCHOR POINT TYPE AND FLOOR MOUNT SLEEVE TYPE SELECTION SHALL BE BASED ON A SITE-BY-SITE BASIS AND PER THE COUNTY'S WRITTEN DIRECTION. PEDESTAL MOUNTED ANCHOR POINTS ARE THE PREFERRED OPTION, HOWEVER; FLUSH MOUNTED ANCHOR POINTS MAY BE USED IF ANCHOR POINT LOCATION, EGRESS/INGRESS, ACCESS TO PORTABLE DIESEL PUMPS, OR OTHER EQUIPMENT IS AN ISSUE; BUT ONLY WITH A CASE BY CASE APPROVAL FROM THE COUNTY.
З.	ALL ANCHOR POINTS MUST BE INSTALLED THE MINIMUM DISTANCE NOTED FROM AN EXISTING EXPANSION JOINT OR FREE EDGE OF CONCRETE. ANCHOR POINTS MAY BE INSTALLED AMONG SAWED JOINTS. TOTAL AREA OF UNINTERRUPTED EXISTING CONCRETE SURROUNDING ANCHOR POINT SHALL BE GREATER THAN OR EQUAL TO THE MINIMUM AREAS STATED IN FALL PROTECTION NOTE 3 ON DRAWING S-05. UNINTERRUPTED CONCRETE IS DEFINED AS A SINGLE POUR OF CONCRETE WITHIN EXPANSION OR CONSTRUCTION JOINTS. SAWED JOINTS ARE PERMITTED WITHIN THE LIMITS OF UNINTERRUPTED CONCRETE.
4.	FALL PROTECTION ANCHORAGE PERIMETER IS THE BOUNDARY AT A MINIMUM DISTANCE FROM THE ANCHORAGE POINT TO THE EDGE OF THE OPENING OF THE WETWELL AND IS ALSO DEPENDENT ON THE LENGTH OF THE FALL PROTECTION LANYARD PROVIDED. FALL PROTECTION ANCHORAGE POINTS SHALL ONLY BE INSTALLED BEYOND THIS PERIMETER. CONTRACTOR TO SET ANCHOR POINT BASED ON FINAL INSTALLATION LOCATION OF LANYARD CONNECTION POINT ON THE HOIST TO ENSURE DISTANCE FROM OPENING TO CONNECTION POINT IS NOT LESS THAN 6 FT. COUNTY STANDARD LANYARD IS 9 FT UNLESS OTHERWISE NOTED.
5. WEW CONCRETE	FALL PROTECTION ANCHOR POINT DESIGN LOADING IS BASED ON THE FALL PROTECTION SLEEVE DESIGN LOADS OF:
	5,000 LBS DOWNWARD FORCE 90,000 IN-LBS OVERTURNING MOMENT
	LOADS ABOVE ARE APPLIED AT THE SLEEVE CONNECTION POINT.
6.	PUMP STATION AND SLAB SHALL BE DESIGNED BY OTHERS.
7.	CONTRACTOR SHALL PAINT ENTIRE PEDESTAL MOUNTED ANCHOR POINT AND A 6" STRIP SURROUNDING THE PERIMETER OF FLUSH MOUNTED ANCHOR POINT SAFETY YELLOW AFTER INSTALLATION.
8.	FOR CAST-IN-PLACE CONCRETE, CONTRACTOR SHALL ROUGHEN EXISTING SURFACE TO 1/4" AMPLITUDE WITH HANDHELD BUSH HAMMER AND PREPARE SURFACE IN ACCORDANCE WITH THE ADHESIVE ANCHOR MANUFACTURER'S RECOMMENDATIONS. EXISTING SURFACE SHALL BE CLEAN AND SOUND CONCRETE. FOR PRECAST PEDESTAL, CONTRACTOR SHALL CLEAN EXISTING CONCRETE SURFACE AND CAREFULLY MARK LOCATIONS OF DOWEL EMBEDMENTS AND SET PRECAST PEDESTAL WITHOUT DAMAGE TO DOWELS.
9.	CONTRACTOR TO INSTALL ANCHOR POINT AT REQUIRED DISTANCE TO MAINTAIN CLEARANCE BETWEEN THE BOOM ARM OF THE HOISTS AND THE ELECTRICAL EQUIPMENT PER LATEST NEC REQUIREMENTS.
IEW CONCRETE	
'E	
EVE	

	GRADE	PLANS FOR PRIVATELY-OWNED PUMPING STATIONS MUST PROVIDE	PLANS FOR PRIVATELY-OWNED PUMPING STATIONS MUST PROVIDE		
WASTEWATER PUMPING STATION DESIGN PARAMETERS	INTERVENTIONS:	THE FOLLOWING INFORMATION AS A MINIMUM REQUIREMENT			
(FOR COUNTY-OWNED STATIONS)	1. CONCRETE (REINFORCED) = 150 lbs./ft. ³				
	INSIDE DIA. = FT. 2. SATURATED SOIL = 120 lbs./ft. ³ INSIDE DIA. = FT. 4. DIMENSIONS IN FEET				
	"B" 5. NO WATER IN WET WELL 6. NEGLECT TOP SLAB WEIGHT	C) SYSTEM vs. PUMP PERFORMANCE CURVE (PLOT ON GRAPH PROVIDED)			
WASTEWATER PUMPING STATION:	U OUTSIDE DIA. = <u>FT.</u> 7. NEGLECT SOIL FRICTION 8. ROUND WETWELL BARREL	D) COPY OF MANUFACTURES PUMP PERFORMANCE CURVES. E) PROVIDE DATA FOR ITEMS (1) THROUGH (11) AS SHOWN BELOW.			
LOCATION: SECTION: TOWNSHIP: RANGE:	부 'A'' 9. CIRCULAR WETWELL BASE 인 4 10. THE WATER TABLE IS AT GRADE	F) PUMP STATION DATA.			
I. SERVICE AREA					
 □ NORTHWEST NAME OF WWTP SERVING THIS DEVELOPMENT: □ CENTRAL □ SOUTH 		(1) TOP SLAB ELEVATION =			
II. DESIGN CAPACITY					
A. AVERAGE DAILY FLOW (A.D.F.):		(2) ELEVATION OF C DISCHARGE PIPING =			
	WETWELL BASE DIA. <u>FT.</u> "C"	(3) INDICATE SIZE, TYPE, & LOCATION OF VALVES AND			
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	(1) BARREL WEIGHT = $(A^2 - B^2)(\pi/4)(E)(150) = [(_)^2 - (_)^2](\pi/4)(_)(150) =$ Ib.	FITTINGS ON DISCHARGE			
B. PEAK INFLUENT RATE (PEAK FACTOR = PER UTILITY TECHNICAL MANUAL, APPENDIX 5)	(1) BARKEE WEIGHT = $(A - B)(\pi/4)(E)(150) = (\)^2 (\pi/4)(\)(150) =$ [D.] (2) BOTTOM SLAB WEIGHT = $(C)^2 (\pi/4)(D)(150) = (\)^2 (\pi/4)(\)(150) =$ [D.]	(4) INVERT ELEVATION =			
PEAK FACTOR = (PER UTILITY TECHNICAL MANUAL, APPENDIX 5)	(3) SOIL WEIGHT = $(C^2 - A^2)(\pi/4)(E)(120-62.4) = [()^2 - ()^2](\pi/4)()(57.6) =$ Ib.	(5) ALARM SIGNAL ELEVATION =			
$\frac{\text{ADF x PEAK FACTOR}}{1440} = \frac{() \times ()}{1440} = \text{G.P.M.}$	(4) TOTAL WEIGHT = (Barrel Weight) + (Bottom Slab Weight) + (Soil Weight) = () + () + () = Ib. (5) WT. OF WATER DISPLACED = [(A) ² (π /4) (E) + (C) ² (π /4) (D)] (62.4)				
C. DESIGN MINIMUM FLOW: $\frac{ADF \times 0.20}{1440} = () \times 0.20 = G.P.M.$	$= [(_)^{2} (\pi / 4) (_) + (_)^{2} (\pi / 4) (_)] (62.4) = $ Ib.				
$\frac{ADF \times 0.20}{1440} = \frac{() \times 0.20}{1440} =$	SAFETY FACTOR = $\frac{\text{TOTAL WEIGHT}}{\text{WT. OF WATER DISPLACED}} = \frac{(4)}{(5)} = \frac{() \text{ Ib.}}{() \text{ Ib.}} = \frac{(1) \text{ Ib.}}{(1) \text{ Ib.}}$	(6) LAG PUMP "ON" ELEVATION = (IF DUPLEX SYSTEM)			
E. VELOCITY IN FORCE MAIN AT MAX. PUMPING RATE = FEET/SECOND	WT. OF WATER DISPLACED (5) () lb.				
III. WETWELL DESIGN (DUPLEX SYSTEM)	SYSTEM vs. PUMP PERFORMANCE CURVE	(7) LEAD PUMP "ON" ELEVATION =			
A. DESIGN CRITERIA: 1. MAXIMUM PUMP MOTOR CYCLE RATE = 6 STARTS PER HOUR		(8) SPECIFY PUMP MAKE,			
2. MAXIMUM DETENTION TIME AT MINIMUM FLOW = 30 MINUTES	PUMP MANUFACTURER: PUMP MODEL: RPM: HP:	(b) OF EON PEON MARKE, MODEL #, H.P.,Q VS TDH, VOLTS/PHASE = (ADD TO P.S.)	DATA BELOW)		
B. PUMP CONTROL LEVEL SETTINGS:	GPM: TDH: IMPELLER DIA./NO.: PHASE: VOLTS: AMPS:	(SEE ALSO C ABOVE)			
1. PUMP CYCLING RATES ARE AT A MAXIMUM WHEN INFLOW EQUALS ONE-HALF THE DESIGN PUMPING RATE OF G.P.M.		(9) PUMP "OFF" ELEVATION =			
2. WETWELL VOLUME REQUIRED BETWEEN LEAD PUMP START AND PUMP SHUT OFF LEVEL = V = CYCLE PERIOD x (1/2) PUMP RATE					
2		(10) WET WELL BOTTOM ELEVATION =			
$V = \frac{10 \text{ MIN. x (1/2) () G.P.M.}}{2} = \underline{\qquad} \text{GALLONS}$		(11) DIAMETER/DIMENSIONS OF WET WELL			
3. WETWELL DIAMETER (D) = FEET					
WETWELL VOLUME = $\frac{\pi(D)^2 \times 7.48 \text{ GAL./C.F.}}{4} = \frac{7.48(\pi)(D)^2}{4} = \frac{7.48(\pi)(m)^2}{4} = \frac{7.48(\pi)(m)^2}{4} = \frac{7.48(\pi)(m)^2}{4} = \frac{1000}{4} = \frac{1000}{4$					
4. WETWELL LEVEL CHANGE BETWEEN PUMP STOP AND LEAD PUMP START =		PUMP STATION DATA (FOR PRIVATELY-OWNED STATIONS)			
(III.B.2GAL.) / (III.B.3 GAL./FT.DEPTH) = FEET DESIGN FOR: INCHES 5. CONTROL ELEVATIONS:		DEVELOPMENT TO BE SERVED: LOCATION (NEAREST CROSS STREET):			
TOP OF SLAB ELEV.		SECTION: TOWNSHIP: RANGE: POWER CO. POLE/PAD NO.:			
INFLUENT INVERT ELEV HIGH WATER ALARM ELEV LAG PUMP ON ELEV	HEAD AND AND AND AND AND AND AND AND AND A	DESIGN PRESSURE AT POINT OF CONNECTION: AVERAGE DAILY FLOW (GPD): PEAK FLOW (GPM):			
LEAD PUMP ON ELEV ALL PUMPS OFF ELEV		P.S.I. x 2.31 = FEET			
BOTTOM ELEV.		WETWELL DIAMETER (FEET): WETWELL VOLUME (GALS./FT. DEPTH): WETWELL DEPTH (FEET):			
IV. SYSTEM CURVE CALCULATIONS					
A. FRICTION LOSS:					
1. PUMPING STATION PIPING					
ITEM SIZE QUANTITY FRICTION TOTAL (INCHES) LOSS (ea.)					
a. TEE x = b. 90° ELBOW x =		NOTES:			
d. GATE VALVE		SYSTEM HEAD VERSUS PUMP PERFORMANCE CURVES ARE TO BE SHOWN TO DETERMINE			
f. WYE X = g. OTHER: X X =		THE SYSTEM PERFORMANCE CAPABILITY AT THE FOLLOWING CONDITIONS:			
g. OTHER: TOTAL = 	FLOW (G.P.M.)	A. CONVENTIONAL PUMPING STATION - FORCE MAIN (NON-MANIFOLD)			
2. FORCE MAIN PIPING:	SYSTEM HEAD COMPUTATION - TOTAL LOSSES IN FEET	1. ONE PUMP RUNNING, IF DUPLEX STATION			
ITEM SIZE QUANTITY FRICTION TOTAL	FLOW RATE (G.P.M.)	2. ONE PUMP AND TWO PUMPS RUNNING, IF TRIPLEX STATION, ETC. 3. IF FORCE MAIN PROFILE RESULTS IN SIPHON, CURVES SHALL SHOW OPERATION AT START-UP			
a. TEE LOSS (ea.) b. 90° ELBOW x	ITEM 0 25 50 75 100 125 150 175 200 225 250 275 300 325 350 375 400 425 450 475 500	(TO HIGH POINT OF PIPING) AS WELL AS FULL FLOW CONDITIONS			
c. CHECK VALVE X = d. GATE VALVE X X =	A. FRICTION LOSS: 1. EQUIVALENT LENGTH	B. MANIFOLDED PUMPING STATIONS			
e. SIDE OUTLET CROSS X = f. WYE X =	PUMPING STATION PIPING: L.F. OF PIPE PIPEPPIPE PIPE PIPEPPIPEPPIPE	ALL CONDITIONS OUTLINED UNDER (A) ABOVE, AND THE FOLLOWING ADDITIONAL CONDITIONS			
g. OTHER: x = TOTAL = "ø PIPE LENGTH =	2. EQUIVALENT LENGTH FORCE MAIN PIPING:	1. SIMULTANEOUS OPERATION OF ALL PUMPING STATIONS ON SYSTEM (WORST CASE) 2. OPERATION WHILE ALL REMAINING STATIONS ARE OFF (BEST CASE)			
(SIZE) (TYPE) TOTAL EQUIVALENT LENGTH =	B. STATIC HEAD:				
B. STATIC HEAD: 1. PIPE CENTER LINE AT DISCHARGE POINT = ELEV FT.	C. DESIGN PRESSURE AT	C. VARIABLE SPEED PUMPING STATIONS ALL APPLICABLE CONDITIONS UNDER (A) AND (B) ABOVE AND IN ADDITION:			
1. PIPE CENTER LINE AT DISCHARGE POINT = ELEV. FT. 2. LOW WATER LEVEL (ALL PUMPS OFF) = ELEV. FT. 3. TOTAL STATIC HEAD (B.1 B.2.) =	POINT OF CONNECTION (BEST/WORST SYSTEM CURVES)	1. OPERATING POINT, INCLUDING SPEED, AT PEAK, AVERAGE, AND MINIMUM FLOWS			
C. PRESSURES AT POINT OF CONNECTION:	D. TOTAL LOSS (FEET):				
BEST & WORST CASE SYSTEM CURVES PROVIDED BY WATER RESOURCE SERVIES, INFRASTRUCTURE PLANNING	(BEST/WORST CASE)				
SCALE REVISIONS	PROJECT No.:	PUMP STATION DESIGN PARAMETERS NOT VALID UNLESS EMBOSSED SEAL SHOWS	HEET		
	FILE No.: HILLSBOROUGH COUNTY	I hereby certify that the work contained herein was prepared under my direct supervision and complies with the requirements of Chapter 471, Florida Statutes and Chapter 61G15, F.A.C.			
	WATER RESOURCE SERVICES DRAWN BY: DOT F. THAYOOD OTDEET (TAMPA EL ODIDA 2000) CHECKED BY:	Signature: Print Name:	OF		
No. DATE DESCRIPTION APPV'D.	925 E. TWIGGS STREET / TAMPA, FLORIDA 33602 ISSUE DATE: OCTOBER 2023 SCALE: NONE	Florida Professional Engineer's Registration Number:	/		





PUMP STATION DATA (FOR PRIVATELY-OWNED STATIONS)					
DEVELOPMENT TO BE SERVED:		LOCATION (NEAREST CROSS STREET):			
SECTION:	TOWNSHIP:	RANGE:	POWER CO. POLE/PAD NO.:		
DESIGN PRESSURE AT POINT OF CONNECTION: P.S.I. x 2.31 = FEET		AVERAGE DAILY FLOW (GPD):		PEAK FLOW (GPM):	
WETWELL DIAMETER (FEET):		WETWELL VOLUME (GALS./FT. DEPTH):		WETWELL DEPTH (FEET):	